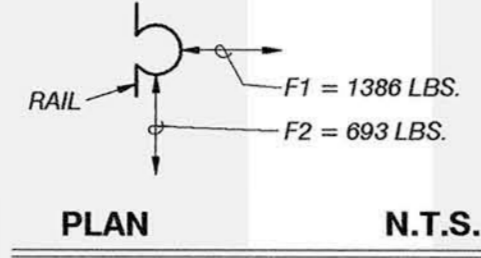


ELEVATOR

THE CONTRACTOR SHALL HAVE DRAWINGS, DESIGN CALCULATIONS AND OTHER PERTINENT DATA PREPARED AND SIGNED BY A CALIFORNIA REGISTERED CIVIL OR STRUCTURAL ENGINEER. THOSE DOCUMENTS SHALL BE REVIEWED BY THE ARCHITECT & STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION AND ARE IN ADDITION TO THOSE DOCUMENTS PREPARED BY JOHN C. FORESTELLE, S.E., INC.

THE ELEVATOR SHAFT & FOUNDATION SYSTEM SHOWN ON THESE DRAWINGS IS DESIGNED AROUND THE PRELIMINARY INFORMATION PROVIDED BY THE MANUFACTURER FOR A "THYSSEN DRUPP" ELEVATOR, MODEL 30 H.L.S. TWIN POST HL. THE FINAL MANUFACTURER DOCUMENTS SHALL BE REVIEWED PRIOR TO CONSTRUCTION.

ELEVATOR CAPACITY = 3000 LBS.



ABBREVIATIONS

ARCH.	= ARCHITECTURAL DRAWINGS	MAX.	= MAXIMUM
B.N.	= BOUNDARY MAILING	MIN.	= MINIMUM
C.J.	= CONTROL JOINT	N.S.	= NEAR SIDE
CLR.	= CLEAR	N.T.S.	= NOT TO SCALE
CONT.	= CONTINUOUS (SPlice ONLY AS DETAIL)	O.C.	= ON CENTER
CTS.K.	= COUNTER SINK	P.T.	= PRESSURE TREATED
d	= PENNY	SHTG.	= SHEATHING
E.J.	= EXPANSION JOINT	SYM.	= SYMMETRICAL
E.N.	= PLYWOOD EDGE NAIL	T.O.P.	= TOP OF PARAPET
(E)	= EXISTING	T. STL.	= TOP OF STEEL
F.F.	= FINISH FLOOR	T.O.W.	= TOP OF WALL
F.G.	= FINISH GRADE	U.N.O.	= UNLESS NOTED OTHERWISE
F.O.C.	= FACE OF CONCRETE	WJ	= WITH
F.O.M.	= FACE OF MASONRY		
F.O.SHTG.	= FACE OF SHEATHING		
F.O.STL.	= FACE OF STEEL		
L.L.H.	= LONG LEG HORIZONTAL		
L.L.V.	= LONG LEG VERTICAL		
[X-Y]	= BOTTOM OF FOOTING ELEVATION		
(*)	= ITEM, DIMENSION OR ELEVATION DEFINED & DESIGNED BY ELEVATOR MANUFACTURER. SEE ELEVATOR MANUFACTURER DRAWINGS & SPECIFICATIONS FOR ELEVATOR COMPONENTS, ANCHORAGE LAYOUT & PROJECTIONS, WALL OPENINGS, DIMENSIONS & OTHER ITEMS NOT SHOWN ON THIS DRAWING.		

SAWN LUMBER

LUMBER SHALL BE DOUGLAS FIR & GRADE MARKED AS FOLLOWS (UNO):
2x6 & DEEPER NO.2
3x6 & LARGER NO.1
SILLS (HEM FIR OR DOUGLAS FIR) (PRESSURE TREATED) NO.2

LUMBER SHALL BE "S-DRY" (EXCEPTION: LUMBER MAY BE SEASONED BEFORE PLACEMENT TO A MAXIMUM MOISTURE CONTENT OF 19% (NOT "S-DRY") IF APPROVED BY THE ENGINEER, VERIFIED BY AN INDEPENDENT TESTING LABORATORY AND DOES NOT IMPAIR THE QUALITY OF WORK)

STANDARD COT WASHERS SHALL BE USED UNDER BOLT HEADS AND NUTS THAT WOULD OTHERWISE BEAR ON WOOD SURFACES. ALL LIGHT GAGE METAL CONNECTORS SHALL BE MANUFACTURED BY "SIMPSON STRONG-TIE CO., INC." OR APPROVED EQUAL. RETIGHTEN BOLTS BEFORE CLOSING IN.

NAILS (UNO) SHALL BE UNCOATED AND HAVE FULL HEADS. WHERE DRIVING NAILS CAUSES SPLITTING, LEAD HOLES WITH A DIAMETER EQUALING 60 TO 75 PERCENT OF THE NAIL DIAMETER SHALL BE PROVIDED. NAILING SHALL BE AS PER C.B.C. "NAILING SCHEDULE."

NAILS IN CONTACT WITH PRESSURE TREATED AND FIRE TREATED WOOD SHALL BE COMMON GAGE WITH FULL HEADS AND BE HOT-DIPPED ZINC COATED GALVANIZED OR STAINLESS STEEL.

LAG BOLT OR LAG SCREW CONNECTORS SHALL BE INSERTED IN DRILLED HOLES WITH SOAP OR OTHER LUBRICANT. THE THREADED PORTION OF THE CONNECTOR SHALL BE PLACED IN ITS LEAD HOLE BY TURNING IT WITH A WRENCH AND NOT BY DRIVING IT WITH A HAMMER.

LEAD HOLES SHALL BE BORED AS FOLLOWS (UNO):
1. CLEARANCE HOLE FOR SHANK = SHANK DIAMETER.
2. LEAD HOLE FOR THREADED PORTION = 80% TO 70% OF SHANK DIAMETER.

PLYWOOD SHEATHING

IDENT.	MATERIAL	NAILING	REMARKS
ROOF SHEATHING	1/2" OR 5/8" THICK PLYWOOD OR OSB, C-D, STRUCT. 1, EXPOSURE 1, 32/16, SIZED FOR SPACING	10d @ 6" BOUND. (B.N.) 10d @ 6" EDGES (E.N.) 10d @ 12" INTERM. (RING OR SCREW SHANK NAILS)	
ROOF TRUSS SHEATHING	1/2" OR 5/8" THICK PLYWOOD OR OSB, C-D, STRUCT. 1, EXPOSURE 1, 32/16, SIZED FOR SPACING	10d @ 6" BOUND. (B.N.) 10d @ 6" EDGES (E.N.) 10d @ 6" INTERM.	

- NOTES:
- PANELS SHALL CONFORM TO PS-1-96. HORIZONTAL PANELS SHALL BE STAGGERED AND PLACED SO THAT THE FACE GRAIN IS PERPENDICULAR TO THE SUPPORTING MEMBERS (UNO). PROVIDE 1/8" SPACING AT PANEL ENDS & EDGES.
 - 10d PLYWOOD NAILS SHALL BE .148" Ø X 2-3/8" LONG.
 - NO PANEL LESS THAN 24" WIDE SHALL BE USED IN ANY HORIZONTAL DIAPHRAGM.
 - THE EDGES OF ALL OPENINGS SHALL BE PERIMETER NAILED PER THE BOUNDARY NAILING REQUIRED FOR THAT AREA.
 - THE PANEL EDGE DISTANCE ON A 1 1/2" OR WIDER MEMBER SHALL BE 3/4" MINIMUM. THE PANEL EDGE DISTANCE ON A 2 1/2" OR WIDER MEMBER SHALL BE 1/2" MINIMUM. NAILS MAY BE SLANT DRIVEN TO MAINTAIN THE MINIMUM EDGE DISTANCE.
 - NAIL HEADS SHALL NOT PENETRATE THE OUTER PLY OF THE PANEL MORE THAN WOULD BE NORMAL FOR A HAND-HAMMER.

EPOXY DOWELS (REGULAR-WEIGHT CONCRETE)

ANCHOR TYPE	DIAMETER INCHES	HOLE SIZE (UNO)	HYDRAULIC RAM TENSION TEST LOAD (LBS.) @ 2.0 X ALLOW.
REINFORCING DOWELS (GRADE 60) (F _c = 2000 PSI)	#4 #5 #6	3/4" Ø X 4 1/2" DEEP 1 1/8" Ø X 5 1/2" DEEP 1 3/4" Ø X 6 1/2" DEEP	NO TESTING NO TESTING NO TESTING

- NOTES:
- (*) INDICATES TEST LOAD IS GOVERNED BY THE YIELD STRENGTH OF THE ANCHOR (.80F_yA₅₀).
 - EPOXY SHALL BE HILTI "HIT HY-150" ADHESIVE ANCHOR SYSTEMS (ICC = ESR-1987)
 - SPECIAL TESTS & INSPECTIONS:
 - 100% OF DOWEL INSTALLATION PROCEDURES SHALL BE INSPECTED. THE INSTALLATION SHALL BE DONE IN THE PRESENCE OF THE SPECIAL INSPECTOR AND COMPLY WITH ITS ICBO REPORT.

EPOXY DOWELS (SOLID GROUTED CONCRETE MASONRY)

ANCHOR TYPE	DIAMETER INCHES	HOLE SIZE (UNO)	DIRECT PULL TEST LOAD (LBS.)
REINFORCING DOWELS (GRADE 60) (F _c = 2000 PSI)	#4 #5 #6	3/4" Ø X 4 1/2" DEEP 1 1/8" Ø X 5 1/2" DEEP 1 3/4" Ø X 6 1/2" DEEP	NO TESTING NO TESTING NO TESTING

- NOTES:
- CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH f_c = 2000 PSI.
 - EPOXY SHALL BE HILTI "HIT HY-150" ADHESIVE ANCHOR SYSTEMS (ICC = ESR-1987)
 - SPECIAL TESTS & INSPECTIONS:
 - 100% OF DOWEL INSTALLATION PROCEDURES SHALL BE INSPECTED. THE INSTALLATION SHALL BE DONE IN THE PRESENCE OF THE SPECIAL INSPECTOR AND COMPLY WITH ITS ICBO REPORT.

EXPANSION ANCHORS (SOLID GROUTED CONCRETE MASONRY)

ANCHOR TYPE	THREAD DIAMETER (INCHES)	EMBED DEPTH INCHES (UNO)	HYDRAULIC RAM TENSION TEST LOAD (LBS.) @ 1.5 X ALLOW.
WEDGE ANCHOR "Kwik Bolt" HILTI INC. ICBO NO. ESR-1385 INTERIOR AREAS	1/4" 3/8" 1/2" 5/8" 3/4"	1 1/2" 1 3/4" 2 1/4" 2 3/4" 3 1/4"	240 510 1000 1300 1650

- NOTES:
- THE INFORMATION GIVEN ABOVE IS FOR CARBON STEEL ANCHORS LOCATED IN INTERIOR AREAS & INSTALLED IN THE FACE SHELLS OF SOLID GROUT-FILLED CONCRETE MASONRY WALLS WITH SPECIAL INSPECTIONS.
 - SPECIAL TESTS & INSPECTIONS:
 - 100% OF ANCHOR & DOWEL INSTALLATION PROCEDURES SHALL BE INSPECTED. THE INSTALLATION SHALL BE DONE IN THE PRESENCE OF THE SPECIAL INSPECTOR AND COMPLY WITH ITS ICBO REPORT.
 - ANCHORS SHALL BE INSTALLED A MINIMUM OF 1 1/2" FROM ANY VERTICAL MORTAR JOINT.
 - ANCHOR LOCATIONS SHALL BE LIMITED TO ONE PER MASONRY CELL WITH A MINIMUM SPACING OF 8" ON CENTER.

CONCRETE

MATERIALS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

PORTLAND CEMENT (ONE BRAND, TYPE V) ASTM C150
WATER DOMESTIC SUPPLY
AGGREGATES (FINE & COARSE) ASTM C33
(AGGREGATES SHALL CONTAIN NOT MORE THAN 2% SHALE OR OTHER DELETERIOUS PARTICLES.)
ADMIXTURE (TYPE A, WATER REDUCING) ASTM C494
FLY ASH ASTM C618 - CLASS F
CURING COMPOUND ASTM C309
(SHALL NOT IMPAIR IN ANY WAY THE APPLICATION OF FINISHES, COVERINGS OR PAINT)

CONTRACTOR SHALL SUBMIT A DESIGN MIX, SPECIFICALLY PREPARED FOR THIS PROJECT AND APPROVED BY A CALIFORNIA LICENSED CIVIL ENGINEER, TO THE ARCHITECT FOR REVIEW 15 WORKING DAYS PRIOR TO PLACEMENT. THE CONCRETE SHALL BE REGULAR WEIGHT (145 LBS.) AND THE MIX SHALL CONFORM TO THE FOLLOWING:
MINIMUM ULTIMATE COMPRESSIVE STRENGTH @ 28 DAYS 3000 PSI
MAXIMUM SLUMP (PER ASTM C-143) 4 INCHES
MAXIMUM AGGREGATE SIZE 3/4" OR 1 INCH
MINIMUM CEMENT CONTENT 5 SACKS/CY
MAXIMUM WATER/CEMENT RATIO50

CONCRETE SLABS SHALL NOT BE PLACED SOONER THAN 24 HOURS AFTER THE CONCRETE FOUNDATIONS HAVE BEEN PLACED. COLD JOINTS SHALL OCCUR WHERE SHOWN ON THE DRAWINGS.

CONCRETE - REGULAR WEIGHT FILL OVER METAL DECK

MATERIALS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

PORTLAND CEMENT (ONE BRAND, TYPE II) ASTM C150
WATER DOMESTIC SUPPLY
AGGREGATES (FINE & COARSE) ASTM C33
(AGGREGATES SHALL CONTAIN NOT MORE THAN 2% SHALE OR OTHER DELETERIOUS PARTICLES.)
ADMIXTURE (TYPE A, WATER REDUCING) ASTM C494
(NO ADMIXTURE SHALL BE USED THAT CONTAINS CHLORIDE, CHLORIDE SALTS OR OTHER DELETERIOUS MATERIAL)
CURING COMPOUND ASTM C309
(SHALL NOT IMPAIR IN ANY WAY THE APPLICATION OF FINISHES, COVERINGS OR PAINT)

CONTRACTOR SHALL SUBMIT A DESIGN MIX, SPECIFICALLY PREPARED FOR THIS PROJECT AND APPROVED BY A CALIFORNIA LICENSED CIVIL ENGINEER, TO THE ARCHITECT FOR REVIEW 15 WORKING DAYS PRIOR TO PLACEMENT. THE CONCRETE SHALL BE REGULAR WEIGHT (145 LBS.) AND THE MIX SHALL CONFORM TO THE FOLLOWING:
MINIMUM ULTIMATE COMPRESSIVE STRENGTH @ 28 DAYS 3000 PSI
MAXIMUM SLUMP (PER ASTM C-143) 4 INCHES
MAXIMUM AGGREGATE SIZE 3/4" OR 1 INCH
MINIMUM CEMENT CONTENT 5 SACKS/CY
MAXIMUM WATER/CEMENT RATIO60

CONCRETE FILL SHALL NOT RECEIVE ANY CONSTRUCTION LOAD UNTIL THE CONCRETE HAS CURED FOR 7 DAYS AND HAS REACHED 75% OF ITS ULTIMATE COMPRESSIVE DESIGN STRENGTH.

CONCRETE MASONRY

CONCRETE BLOCK SHALL BE GRADE N-1, MEDIUM OR LIGHT WEIGHT, OPEN-END UNITS AND CONFORM TO ASTM C-90. FACE TEXTURE AND COLOR SHALL BE AS DEFINED BY THE ARCHITECT.

FILL ALL CELLS WITH GROUT. ALL OVERHANGING MORTAR AND MORTAR DROPPINGS SHALL BE REMOVED. ALL ISOLATED BOLTS EMBEDDED IN MASONRY SHALL BE GROUTED SOLIDLY IN PLACE WITH NOT LESS THAN 1" OF GROUT SURROUNDING THE BOLT. IF WORK IS STOPPED FOR ONE HOUR OR LONGER, PROVIDE A HORIZONTAL CONSTRUCTION JOINT BY STOPPING THE GROUT 1-1/2" BELOW THE TOP OF THE BLOCK.

CONSTRUCTION SHALL COMPLY WITH CBC SECTION 2104 AND BE ONE OF THE TWO FOLLOWING METHODS:

- LOW-LIFT GROUTED CONSTRUCTION:
 - GROUT LIFTS AND POURS SHALL NOT EXCEED 4'-0".
 - VERTICAL REINFORCING SHALL BE WIRED INTO POSITION AT SPLICES AND AT INTERVALS NOT EXCEEDING 200 BAR DIAMETERS.
- HIGH-LIFT GROUTED CONSTRUCTION:
 - GROUT LIFTS SHALL NOT EXCEED 4'-0".
 - GROUT POURS GREATER THAN 8'-0" SHALL RECEIVE CONTINUOUS SPECIAL INSPECTIONS PER CBC SECTION 1701. THE SPECIAL INSPECTOR SHALL BE SELECTED BY THE OWNER AND APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.
 - CLEANOUTS SHALL BE PROVIDED AT THE BOTTOM COURSE OF EVERY VERTICAL BAR. THE BOTTOM COURSE OF THE POUR SHALL BE CONSTRUCTED ENTIRELY OF INVERTED OPEN END BOND BEAM UNITS. CLEANOUTS SHALL BE SEALED AFTER SPECIAL INSPECTIONS AND BEFORE GROUTING.
 - GROUT SHALL CONTAIN "SIKA GROUT AID" AS PER THE MANUFACTURER'S RECOMMENDATIONS.
 - VERTICAL REINFORCING SHALL BE WIRED INTO POSITION AT SPLICES AND AT INTERVALS NOT EXCEEDING 200 BAR DIAMETERS FOR BOTH CONSTRUCTION METHODS SPECIFIED BELOW.

CONTRACTOR SHALL SUBMIT A GROUT DESIGN MIX, SPECIFICALLY PREPARED FOR THIS PROJECT AND APPROVED BY A CALIFORNIA LICENSED CIVIL ENGINEER, TO THE ARCHITECT FOR REVIEW 15 WORKING DAYS PRIOR TO PLACEMENT. MORTAR & GROUT MIXES SHALL CONFORM TO THE FOLLOWING:
MINIMUM MORTAR STRENGTH @ 28 DAYS (TYPE S) 1800 PSI (ASTM C-270)
MINIMUM COARSE GROUT STRENGTH @ 28 DAYS 2000 PSI (ASTM C-470)

STEEL

MATERIALS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

WIDE FLANGE SHAPES ASTM A992 (F_y = 36 KSI)
CHANNELS, PLATES, BARS, ANGLES & THRD RODS ASTM A36 (F_y = 36 KSI)
PIPES ASTM A53, GRADE B, (F_y = 35 KSI)
SQUARE & RECTANGULAR TUBING ASTM A500, GRADE B, (F_y = 40 KSI)
COMMON BOLTS ASTM A307, GRADE B
HIGH STRENGTH BOLTS ASTM A325X
ANCHOR RODS ASTM F1554 GRADE 36
WELDING ELECTRODES AWS E70XX
AUTOMATIC END WELDED STUDS ASTM A108
(NELSON GRANULAR FLUX FILLED STUDS C1015 COLD ROLLED STEEL)

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND REVIEWED BY THE ENGINEER PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BE APPROVED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT. THE STEEL FABRICATOR SHALL VERIFY ALL DIMENSIONS, DETAILS AND QUANTITIES WITH THE DRAWINGS AND EXISTING CONDITIONS.

FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF THE AISC SPECIFICATIONS. NO MEMBER SHALL BE SPLICED UNLESS DETAILED IN THE DRAWINGS. BOLTED CONNECTIONS SHALL BE ACCOMPLISHED WITH DRILLED OR PUNCHED HOLES. STEEL NOT IN CONTACT WITH CONCRETE SHALL RECEIVE ONE COAT OF SHOP PAINT IN ACCORDANCE WITH THE PROVISIONS OF THE CODE OF STANDARD PRACTICE OF AISC AND THE PROJECT SPECIFICATIONS. ADDITIONAL PAINTING OF EXPOSED STEEL SHALL BE AS DIRECTED BY THE ARCHITECT.

SPECIAL TESTS AND INSPECTIONS OF MATERIAL AND SHOP & FIELD WELDING OPERATIONS SHALL COMPLY WITH THE CBC CHAPTER 17 AND THE "TESTS AND INSPECTIONS" SCHEDULE. WELDERS SHALL BE CERTIFIED AND WORK DONE IN ACCORDANCE WITH THE AWS WELDING CODE. THE SPECIAL INSPECTOR SHALL BE SELECTED BY THE OWNER AND APPROVED BY THE ARCHITECT AND ENGINEER. THE CONTRACTOR SHALL COORDINATE SCHEDULING TIMES FOR TESTS AND INSPECTIONS.

METAL DECKING

MATERIALS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

DECKING ASTM A 653 (F_y = 33 KSI)
CLOSURES & ACCESSORIES ASTM A 653 (F_y = 33 KSI)
GALVANIZING ASTM A 653 (G-60)

FABRICATION AND ERECTION SHALL BE AS RECOMMENDED BY AN APPROVED MANUFACTURER AND SHALL COMPLY WITH THE PROVISIONS OF THE STEEL DECK INSTITUTE'S DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS.

METAL DECK SHALL BE 1-1/2" X 16 GAGE, "B-36" DECK AS MANUFACTURED BY "VERCO MANUFACTURING CO." END OF DECK MAY BE LAPPED OR BUTTED.

SIDELAP: BUTTON PUNCH @ 24" O.C. OR TOP SEAM WELD @ 24" O.C.
TRANSVERSE: 3/4" Ø PLUG WELD (36-5 PATTERN) TO MEMBERS PERP. TO FLUTES
LONGITUDE: 3/4" Ø PLUG WELDS @ 12" O.C. TO MEMBERS PARALLEL TO FLUTES

GENERAL

PERTINENT DETAILS AND GENERAL NOTES SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY SHOWN OTHERWISE ON THE DRAWINGS. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER SIMILAR WORK. NO DEVIATIONS FROM THE DRAWINGS SHALL BE PERMITTED WITHOUT PERMISSION OF THE ENGINEER. SEE NON-STRUCTURAL DRAWINGS AND SPECIFICATIONS FOR ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL EQUIPMENT SHALL BE BRACED AND ANCHORED ADEQUATELY FOR SEISMIC LOADS. WHERE CONSTRUCTION MATERIALS ARE TEMPORARILY STORED ON THE ROOF FRAMING, THEY SHALL BE DISTRIBUTED SO THAT THE LOAD DOES NOT EXCEED 20 LBS/SF.

THE CONTRACTOR SHALL:

- VERIFY ALL (E) EXISTING CONDITIONS AND DIMENSIONS AND REPORT ANY DIFFERENCES IN ASSUMED CONDITIONS TO THE ENGINEER PRIOR TO PROCEEDING.
- PROVIDE AND BE SOLELY RESPONSIBLE FOR ALL TEMPORARY SHORING, GUYS AND BRACING NECESSARY FOR STRUCTURAL WORK & PLACEMENT OF EQUIPMENT SHOWN ON THESE DRAWINGS.
- PERFORM ALL WORK AND RECEIVE ALL TESTS AND INSPECTIONS IN ACCORDANCE WITH THE PROJECT DRAWINGS, SPECIFICATIONS, THE "STRUCTURAL TESTS AND INSPECTIONS" SCHEDULE AND THE 2007 C.B.C.
- BE SOLELY RESPONSIBLE FOR CONDITIONS AT THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY.
- PERFORM ALL DEMOLITION, REMOVAL AND DISPOSAL IN CONFORMANCE WITH ALL APPLICABLE LAWS, ORDINANCES, RULES AND REGULATIONS. EXCESS MATERIAL SHALL BE REMOVED FROM THE SITE AND DISPOSED OF OFF SITE UNLESS OTHERWISE DIRECTED BY THE ARCHITECT.

SUPPORT SERVICES PERFORMED BY THE ENGINEER DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES THAT ARE FURNISHED BY OTHERS. THE SUPPORT SERVICES PERFORMED BY THE ENGINEER ARE PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH THE CONTRACT DRAWINGS AND SPECIFICATIONS. THESE SUPPORT SERVICES DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

EXISTING (E) DIMENSIONS ARE APPROXIMATE AND ARE GIVEN FOR THE CONTRACTOR'S CONVENIENCE ONLY. DIMENSIONS AND EXISTING CONDITIONS SHALL BE VERIFIED AT THE SITE BY THE CONTRACTOR PRIOR TO CONSTRUCTION.

ITEMS SHOWN ON THESE DRAWINGS ARE NEW CONSTRUCTION UNLESS IDENTIFIED AS (E) EXISTING. NEW STRUCTURAL MATERIAL SHALL BEAR DIRECTLY ON (E) CONCRETE OR MASONRY (UNO). (E) NON-STRUCTURAL MATERIALS TO BE REMOVED SHALL BE REPLACED OR PATCHED PER THE ARCHITECTURAL DRAWINGS.

NOTIFICATION

THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER TWO (2) WORKING DAYS PRIOR TO:

- DEMOLITION.
- START OF CONSTRUCTION.
- PLACING CONCRETE, MASONRY & GROUT.
- COVERING METAL DECKING & STEEL FRAMING.

DESIGN CRITERIA (2007 C.B.C.)

EARTHQUAKE DESIGN:
IMPORTANCE FACTOR I = 1.00
OCCUPANCY CATEGORY = II
SITE CLASS = D
MAPPED SPECTRAL RESPONSE ACCELERATIONS
S_s = 1.017 S₁ = .428
SPECTRAL RESPONSE COEFFICIENTS,
S_{ds} = .815 S_{d1} = .448
SEISMIC DESIGN CATEGORY = D
BASIC SEISMIC SYSTEM "BUILDING"
BUILDING FRAME SYSTEM (ORDINARY REINFORCED MASONRY SHEAR WALLS)
ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE
RESPONSE MODIFICATION FACTOR R = 2.5
SYSTEM OVERSTRENGTH FACTOR = 2.0
DEFLECTION AMPLIFICATION FACTOR C_d = 1.75

WIND DESIGN:
BASIC WIND SPEED = 85 MPH
IMPORTANCE FACTOR I = 1.00
EXPOSURE = C

ROOF & FLOOR LOADS:
ROOF LIVE LOAD 20 LBS/SF (REDUCIBLE WHERE ALLOWED BY CODE);
DEAD LOADS SHALL BE BASED ON THE CONDITIONS SHOWN ON THE DRAWINGS.

FLOOR LIVE LOAD 50 LBS/SF (PLUS 20 LBS/SF PARTITION LOAD)
FLOOR LIVE LOAD 100 LBS/SF AT LANDINGS
DEAD LOADS SHALL BE BASED ON THE CONDITIONS SHOWN ON THE DRAWINGS.

THE INTENT OF THE DRAWINGS AND SPECIFICATIONS IS TO CONSTRUCT THE NEW WORK IN ACCORDANCE WITH THE 2007 C.B.C. SHOULD ANY CONDITION DEVELOP THAT IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE NEW WORK WILL NOT COMPLY WITH THESE REGULATIONS, A CHANGE ORDER SHALL BE SUBMITTED TO AND APPROVED BY THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.

THE SCOPE OF STRUCTURAL DESIGN SERVICES DOES NOT INCLUDE A STRUCTURAL EVALUATION OR ANALYSIS OF THE EXISTING BUILDING. THE STRUCTURAL WORK SHOWN ON THESE DOCUMENTS DOES NOT SIGNIFICANTLY ALTER THE EXISTING STRUCTURAL INTEGRITY OF THE BUILDING. THE BUILDING WAS BUILT ABOUT 1948 WITH ADDITIONS IN 1955 AND 1981. THEREFORE PORTIONS OF THE BUILDING MAY NOT MEET THE CURRENT CODE REQUIREMENTS FOR EARTHQUAKE AND WIND DESIGN.

FOUNDATION

FOUNDATIONS AND SLABS SHALL BE PLACED ONLY ON FIRM BEARING AND NOT ON UNCOMPACTED FILL. THE CONTRACTOR SHALL PERFORM WORK AND RECEIVE TESTS & INSPECTIONS IN ACCORDANCE WITH THE DRAWINGS, SPECIFICATIONS AND THE SOILS REPORT BY SOILS ENGINEERING, INC., FILE NO. 08-12408, DATED APRIL 18, 2008. THE GEOTECHNICAL ENGINEER SHALL APPROVE SUBGRADES PRIOR TO THE PLACEMENT OF REINFORCING STEEL.

SOIL TYPE SAND & SILTY SAND
BASIC ALLOWABLE BEARING PRESSURE 2000 LBS/SF

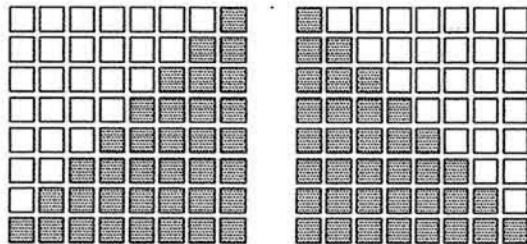
EXCAVATIONS SHALL COMPLY WITH "OSHA" STANDARDS AND THE SOILS REPORT.

REINFORCING

MATERIALS SHALL BE CONTINUOUS, DEFORMED BARS CONFORMING TO ASTM A615.
#3 BARS SHALL BE GRADE 40 OR GRADE 60.
#4 BARS & LARGER SHALL BE GRADE 60.

CONTINUOUS BAR SPLICES SHALL BE STAGGERED AT 4'-0" O.C. AND BE:
CONCRETE 80 BAR DIAMETERS (2'-0" MIN.)
MASONRY 48 BAR DIAMETERS (2'-0" MIN.)

CONCRETE COVERAGE FOR REINFORCEMENT SHALL BE:
CAST AGAINST EARTH & PERMANENTLY EXPOSED TO EARTH 3"
CAST AGAINST FORMS & PERMANENTLY EXPOSED TO EARTH 2"
OR WEATHER
CAST AGAINST FORMS & NOT PERMANENTLY EXPOSED TO EARTH OR WEATHER:
SLABS, WALLS & JOISTS 3/4"
BEAMS, GIRDERS & COLS. 1-1/2"



ORDIZ
MELBY
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DANNY E. ORDIZ, AIA
ARCHITECT C-14,728
WILLIAM J. MELBY, AIA
ARCHITECT C-16,835
CONSULTANT



1.12.09

ELEVATOR ADDITION
FOR:

AJITPAL TIWANA, M.D.

2700 "F" STREET
BAKERSFIELD
CALIFORNIA
93301

NOT FOR
CONSTRUCTION

MARK	DATE	DESCRIPTION

JOB NUMBER:
2004759-02
CAD DRAWING FILE:
DRAWN BY:
JQF
CHECKED BY:
JQF
CHECK AND VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH THE WORK. REPORT DISCREPANCIES TO THE ARCHITECT.
THE DRAWINGS, IDEAS, AND DESIGNS REPRESENTED ON THIS SHEET ARE THE PROPERTY OF THE ARCHITECT.
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SHEET TITLE

GENERAL NOTES

SHEET IDENTIFICATION NUMBER

S-101
SHEET OF 5

JQF # 207-053